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ENCE 3610

Soil Mechanics

Lecture 7
Empirical and Field Tests for Hydraulic
Conductivity
Dewatering

Typical Permeability Values

Table 5-10

Typical permeability values in soils (after Carter and Bentley, 1991)

	10 ⁻¹¹	10 ⁻¹⁰	10 ⁻⁹	10 ⁻⁸	10 ⁻⁷	10 ⁻⁶	10 ⁻⁵	10 ⁻⁴	10 ⁻³	10 ⁻²	10 ⁻¹	1
Coefficient of permeability (log scale)												
	m/s											
	cm/s											
Permeability:	Practically impermeable		Very low		Low		Medium		High			
Drainage conditions:	Practically impermeable			Poor			Good					
Typical soil groups:	GC → GM →		SM		SW →		GW →					
	CH SC SM-SC		MH		SP →		GP →					
	MC-CL											
Soil types:	Homogeneous clays below the zone of weathering		Sils, fine sands, silty sands, glacial till, stratified clays			Clean sands, sand and gravel mixtures			Clean gravels			
			Fissured and weathered clays and clays modified by the effects of vegetation									

Note: The arrow adjacent to group classes indicates that permeability values can be greater than the typical value shown.

Table 5-11

Typical permeability values for highway materials (after Krebs and Walker, 1971)

Materials	Permeability (cm/sec)
Uniformly graded coarse aggregate	40 - 4x10 ⁻¹
Well-graded aggregate without fines	4x10 ⁻¹ - 4x10 ⁻³
Concrete sand, low dust content	7x10 ⁻² - 7x10 ⁻⁴
Concrete sand, high dust content	7x10 ⁻⁴ - 7x10 ⁻⁶
Silty and clayey sands	10 ⁻⁵ - 10 ⁻⁷
Compacted silt	7x10 ⁻⁶ - 7x10 ⁻⁸
Compacted clay	Less than 10 ⁻⁷
Bituminous concrete (new pavements)*	4x10 ⁻³ - 4x10 ⁻⁶
Portland cement concrete	less than 10 ⁻⁸

* Values as low as 10⁻⁸ have been reported for sealed, traffic compacted highway pavement.

Empirical Correlations and Typical Values Granular Soils

- Based on tests bolstered by a varying amount of theory
- Hazen's Correlation

$$k = CD_{10}^2$$
 - k in cm/sec
 - $0.8 < C < 1.2$
(typically $C = 1$)
 - Limited to soils with $0.1 \text{ mm} < D_{10} < 3 \text{ mm}$ and $C_u < 5$
- Typical values
- Assume laminar flow in soil

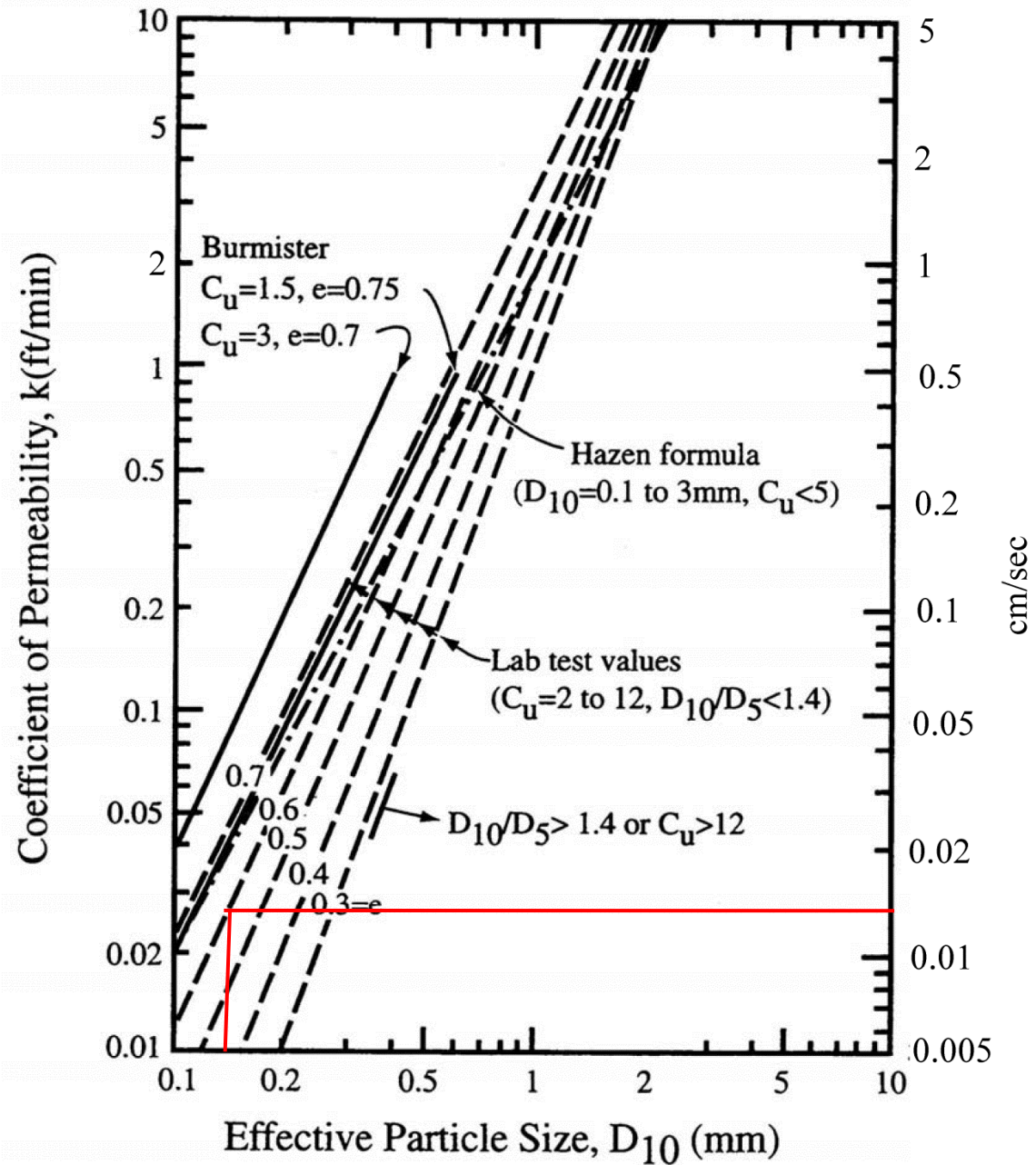


Figure 5-26. The permeability of sands and gravels (after NAVFAC, 1986a).

Note: In Figure 5-26, correlations shown are for remolded compacted sands and sand-gravel mixtures with C_u values as indicated.

Example of Conductivity Estimate

■ Given: Soil as Shown Below

- $D_{10} = 0.15$
- $D_5 = 0.08$
- Uniformity Coefficient
 $C_u = 8$
- Void Ratio $e = 0.6$

■ Find

- Hydraulic Conductivity, using a variety of methods

■ Methods to be used if applicable)

- Hazen's Correlation
- Laboratory Tests Chart

■ Solution

■ Hazen's Correlation

- $0.1 < D_{10} < 5$ is true, but cannot use since $C_u = 8 > 5$

■ Lab Tests

- Can use since $2 < C_u = 12$ and $D_{10}/D_5 = 1.88 > 1.4$
- For $D_{10} = 0.1$ and $e = 0.6$, $k = 0.015$ cm/sec = 0.05 fpm (see chart on previous slide)

Field Permeability Tests and Dewatering Wells

- Field permeability tests measure the coefficient of permeability (hydraulic conductivity) of in-place materials.
- The area and length factors are often combined in a "shape factor" or "conductivity coefficient."
- Measurement of permeability is highly sensitive to both natural and test conditions.
- Field permeability tests are also the "inverse" of the pumping problem for dewatering.

Test types parallel lab tests

Constant Head Test

Variable Head Tests

Rising Head Test

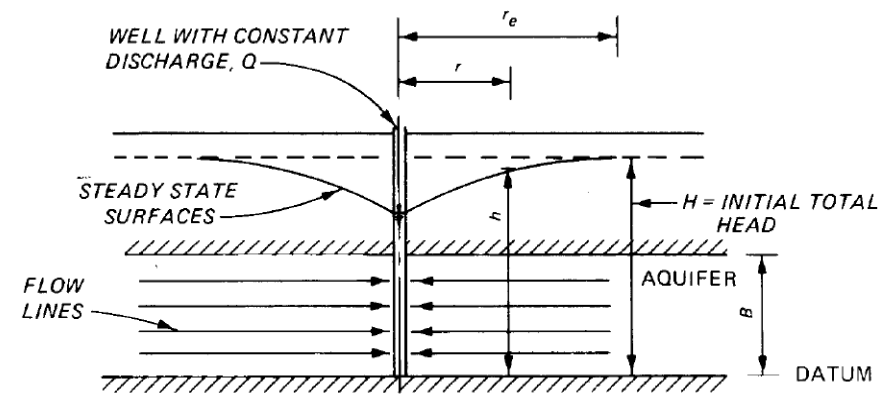
Falling Head Test

Infiltration Test

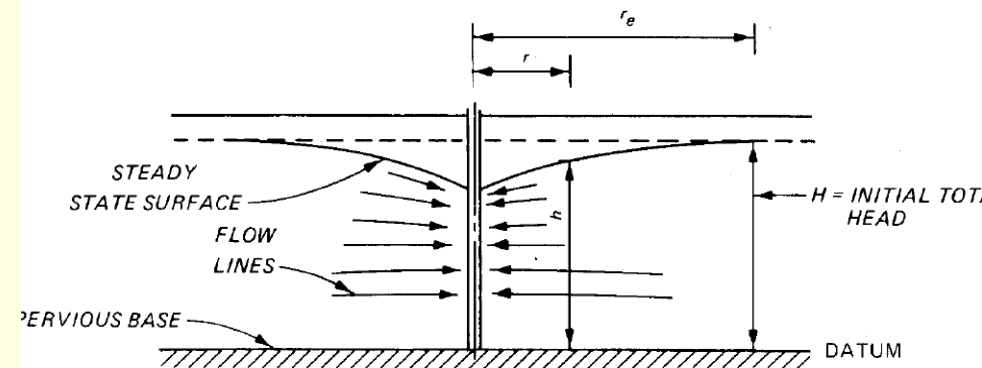
Pumping Test

Gravity and Pressure
Tests

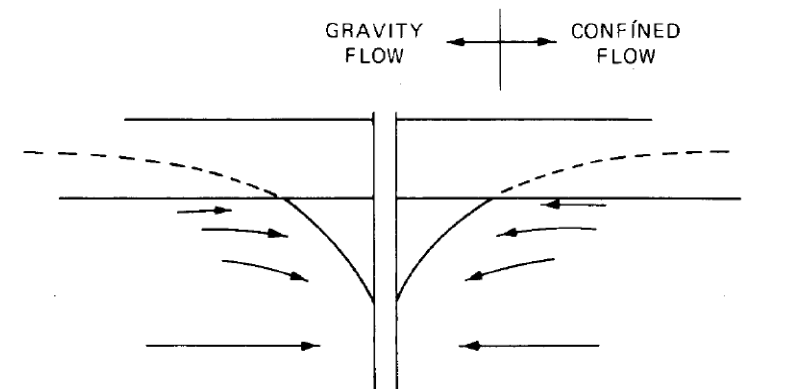
Confined and Unconfined Aquifers



a. HORIZONTAL AQUIFER CONFINED BETWEEN IMPERVIOUS STRATA (ARTESIAN FLOW)



b. HORIZONTAL UNCONFINED AQUIFER (GRAVITY FLOW)



c. COMBINED AND CONFINED AND GRAVITY FLOW

Confined Aquifer: Determination of Height

- Governing Equation:
 - h =height from bottom of well to water level at well
 - h_0 =water table height from bottom of well to water table
 - Q_0 =flow from well
 - k =coefficient of permeability
 - H =thickness of permeable layer
 - r =radius of well or wherever h is measured
 - R =outer boundary of influence of well, or wherever h_0 is measured

$$h = h_0 + \frac{Q_0}{2\pi kH} \ln\left(\frac{r}{R}\right)$$

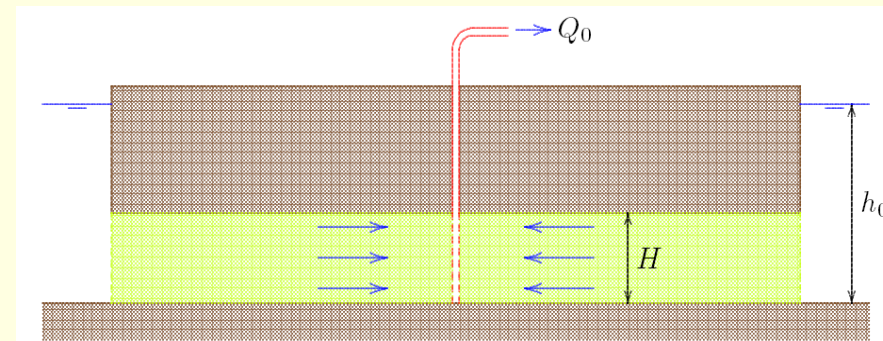


Figure 11.1: Single well in aquifer.

$$Q_0 = \frac{2\pi kH}{\ln\left(\frac{r}{R}\right)} (h - h_0)$$

$$k = \frac{Q_0 \ln\left(\frac{r}{R}\right)}{2\pi H (h - h_0)}$$

Unconfined Aquifer

- Only difference is that upper layer is the same (pervious) as lower layer
- Equations have same variables, but expression is different:

$$h^2 = h_0^2 + \frac{Q_0}{\pi k} \ln\left(\frac{r}{R}\right)$$

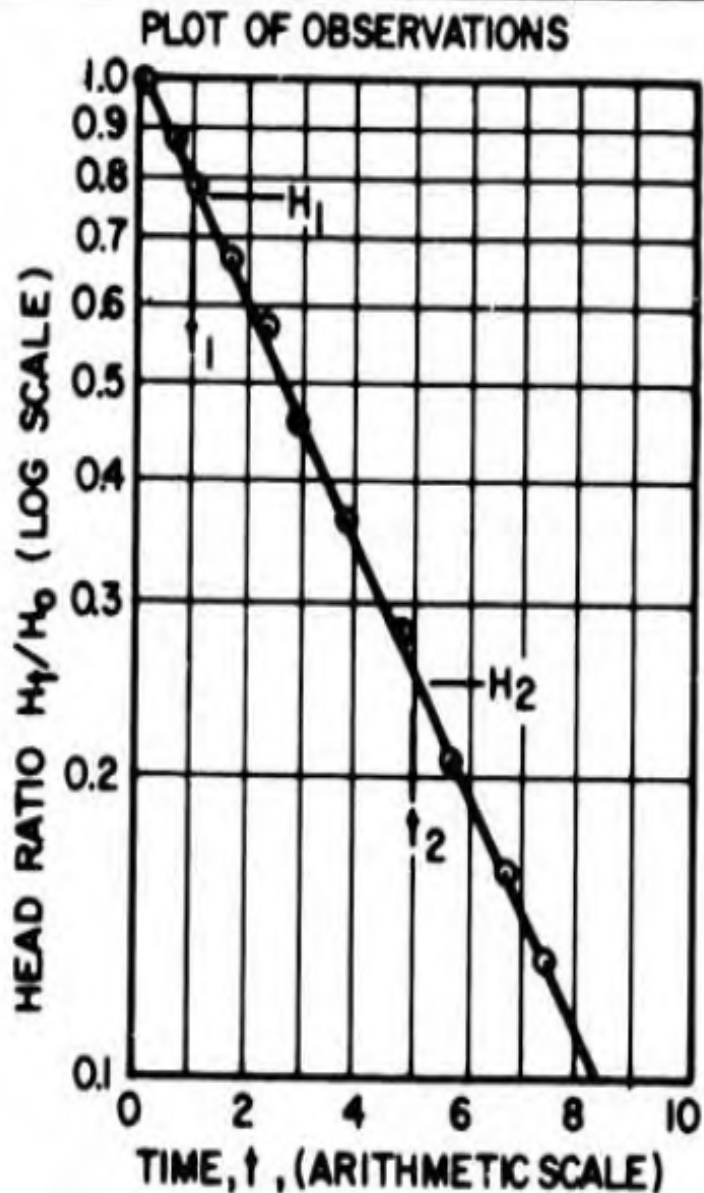
$$Q_0 = \frac{\pi k}{\ln\left(\frac{r}{R}\right)} (h^2 - h_0^2)$$

$$k = \frac{Q_0 \ln\left(\frac{r}{R}\right)}{\pi (h^2 - h_0^2)}$$

Dewatering



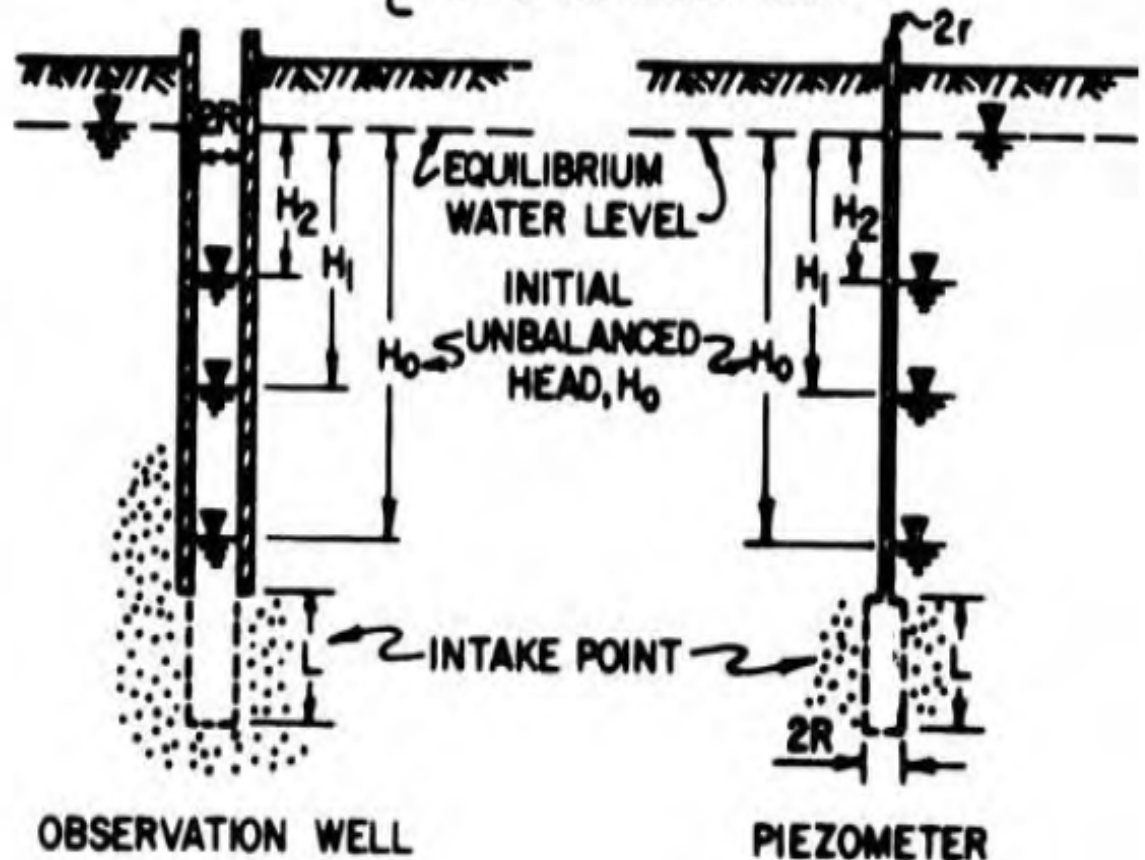
Measuring Permeability in Cohesive Soils (Rising Head Test)



IN GENERAL:

$$K = \frac{A}{F(t_2 - t_1)} \ln\left(\frac{H_1}{H_2}\right)$$

F = SHAPE FACTOR OF INTAKE POINT
 A = STANDPIPE AREA
 K = MEAN PERMEABILITY
 $\ln(H_1/H_2)$ AND $(t_2 - t_1)$ ARE OBTAINED FROM PLOT OF OBSERVATIONS.



Layered Soils

- “Resistor Model” except that we’re dealing in conductivities, not resistances
- For horizontal layers:
 - Vertical flow is a single flow through several hydraulic resistances in series

$$k_{V(eq)} = \frac{H}{\sum_1^n \frac{H_n}{k_{V_n}}}$$

- Horizontal flow is several flows through several hydraulic resistance in parallel

$$k_{H(eq)} = \frac{1}{H} \sum_1^n k_{H_n} H_n$$

Anisotropy in Permeability

$$k_H \neq k_V \text{ (Anisotropic Soil)}$$

$$k_H > k_V \text{ (in general)}$$

- Can be significant, esp. with some types of clays, esp. varved clays
 - Varved clay: “alternating seams or layers of sand, silt and clay” (SFH Table 4.7)
 - Result of sediment deposits with vary in nature due to seasonal fluctuations, etc.

Questions?

